

## AAC anchor X-Pansion internal thread FPX-I (minimum screw property class 4.8)

Highest permissible loads<sup>1)</sup> in aerated concrete

For the design the complete approval ETA - 12/0456 has to be considered.

Type		M6	M8	M10	M12
Minimum member thickness with drill hole cleaning	$h_{\min}$ [mm]	100			
Minimum member thickness without drill hole cleaning	$h_{\min}$ [mm]	120			
Effective anchorage depth	$h_{\text{ef}}$ [mm]	70			
Maximum fastening torque for fixing screw	$T_{\max}$ [Nm]	3,0 <sup>5)</sup>			
<b>Permissible load for single anchors <math>F_{\text{perm}}^{3)}</math></b>					
Min. distance to joints for single anchors	$c_F$ [mm]	0 <sup>9)</sup> / 75 <sup>13)</sup> / 125 <sup>14)</sup>			
Min. edge distance <sup>2)</sup>	$c_1$ [mm]	125 <sup>11)</sup>			
Min. spacing <sup>2)</sup> orthogonal $c_1$	$c_2$ [mm]	188			
Min. spacing <sup>15)</sup>	$a$ [mm]	375 (600) <sup>12)</sup>			
<b>AAC masonry<sup>4)7)</sup></b>	$f_{\text{ck}} \geq 1,6 \text{ N/mm}^2$ $\rho_m \geq 0,25 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	0,3		
	$f_{\text{ck}} \geq 2,0 \text{ N/mm}^2$ $\rho_m \geq 0,35 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	0,4		
	$f_{\text{ck}} \geq 4,0 \text{ N/mm}^2$ $\rho_m \geq 0,50 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	0,9		
	$f_{\text{ck}} \geq 6,0 \text{ N/mm}^2$ $\rho_m \geq 0,65 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	1,4		
<b>AAC slabs<sup>4)</sup>, cracked</b>	$f_{\text{ck}} \geq 3,3 \text{ N/mm}^2$ $\rho_m \geq 0,50 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	0,6		
	$f_{\text{ck}} \geq 4,4 \text{ N/mm}^2$ $\rho_m \geq 0,55 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	0,8		
<b>AAC slabs<sup>4)</sup>, non-cracked</b>	$f_{\text{ck}} \geq 3,3 \text{ N/mm}^2$ $\rho_m \geq 0,50 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	0,8		
	$f_{\text{ck}} \geq 4,4 \text{ N/mm}^2$ $\rho_m \geq 0,55 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	1,2		
<b>Permissible load for anchor groups with 2 or 4 anchors <math>F_{\text{perm},n}^{3)6)8)}</math></b>					
Min. spacing <sup>2)</sup> within a anchor group and 2 single anchors <sup>15)</sup>	$s_{\min}$ [mm]	100			
Min. edge distance <sup>2)</sup>	$c_1$ [mm]	250			
Min. spacing <sup>2)</sup> orthogonal $c_1$	$c_2$ [mm]	375			
Min. spacing	$a$ [mm]	750			
<b>AAC masonry<sup>4)7)10)</sup></b>	$f_{\text{ck}} \geq 1,6 \text{ N/mm}^2$ $\rho_m \geq 0,25 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	0,6		
	$f_{\text{ck}} \geq 2,0 \text{ N/mm}^2$ $\rho_m \geq 0,35 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	0,8		
	$f_{\text{ck}} \geq 4,0 \text{ N/mm}^2$ $\rho_m \geq 0,50 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	1,8		
	$f_{\text{ck}} \geq 6,0 \text{ N/mm}^2$ $\rho_m \geq 0,65 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	2,8		
<b>AAC slabs<sup>4)10)</sup>, cracked</b>	$f_{\text{ck}} \geq 3,3 \text{ N/mm}^2$ $\rho_m \geq 0,50 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	1,2		
	$f_{\text{ck}} \geq 4,4 \text{ N/mm}^2$ $\rho_m \geq 0,55 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	1,6		
<b>AAC slabs<sup>4)10)</sup>, non-cracked</b>	$f_{\text{ck}} \geq 3,3 \text{ N/mm}^2$ $\rho_m \geq 0,50 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	1,6		
	$f_{\text{ck}} \geq 4,4 \text{ N/mm}^2$ $\rho_m \geq 0,55 \text{ kg/dm}^3$	$F_{\text{perm}}^{3)}$ [kN]	2,4		

<sup>1)</sup> The required partial safety factors for material resistance as well as a partial safety factor for load actions of  $\gamma_L = 1,4$  are considered.

<sup>2)</sup> Minimum possible axial spacing resp. edge distance without reducing the permissible load.

<sup>3)</sup> Valid for tensile load, shear load and oblique load under any angle.

<sup>4)</sup> Strength class  $f_{\text{ck}}$  and dry density  $\rho_m$  according EN 771-4 resp. EN 12602.

<sup>5)</sup> If the anchor cannot support against the fixture no installation torque must be applied ( $T_{\max} = 0$ ).

<sup>6)</sup> While using 4 anchors they have to be arranged rectangularly.

<sup>7)</sup> For masoned joints a proof against pull-out of the block is required.

<sup>8)</sup> Permissible total load of the anchor group.

<sup>9)</sup> For joints completely filled with mortar with a joint width  $\leq 12\text{mm}$  and a compressive strength according to EN 998-2  $\geq f_{\text{ck}}$  AAC no distances to joints is required.

<sup>10)</sup> For not visible joints the permissible total load of the anchor group has to be halved and must be designed for multiple use according ETAG 001, Part 6.

<sup>11)</sup> For reinforced AAC slabs with width  $\leq 700\text{mm}$ :  $c_1 \geq 150\text{mm}$ .

<sup>12)</sup> Value in brackets valid for AAC slabs.

<sup>13)</sup>  $c_F$  for tensile load and/or shear load parallel to the joint which is not filled with mortar with width  $\leq 2\text{mm}$ .

<sup>14)</sup>  $c_F = c_1$  for shear load or oblique load orthogonal to the joint which is not filled with mortar with width  $\geq 0\text{mm}$ .

<sup>15)</sup> For 2 single anchors with a spacing  $\leq 375\text{mm}$  ( $\geq s_{\min}$ ) the spacings and edge distances for anchor groups are valid.